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Adaptive Mixed Finite Element Method for Reissner-Mindlin Plates

We use a modified mixed finite element method for the Reissner-Mindlin plate model to study its numerical properties in practical use. We derive an a posteriori error estimate to control adaptive mesh-refining algorithms and study the question of reliability. Numerical examples prove the new scheme efficient.

1. Mechanical Model and Finite Element Discretisation

Due to Reissner-Mindlin theory the deformation vector of a plate Ω with small thickness t and only transverse load f contains three independent components, the rotations $\vartheta = (\vartheta_x, \vartheta_y) \in H_0^1(\Omega)^2$ and the transverse displacement $w \in H_0^1(\Omega)$. The standard Reissner-Mindlin variation formulation is – because of the shear locking phenomena – not sufficient for effective finite element discretisation. So we reformulate the problem with an additional variable [1]

$$\gamma = (\frac{1}{t^2} - \alpha)(\nabla w - \vartheta) \qquad 0 < \alpha < t^{-2}$$
(1)

where α is a parameter to stabilize the discretisation and with the bilinear forms

$$a(w,\vartheta;v,\varphi) := \int_{\Omega} \varepsilon(\vartheta) : \mathbb{C}\varepsilon(\varphi) \, dx + \alpha \int_{\Omega} (\nabla w - \vartheta) \cdot (\nabla v - \varphi) \, dx \tag{2}$$

$$b(w,\vartheta;\eta) := \int_{\Omega} (\nabla w - \vartheta) \cdot \eta \, dx \tag{3}$$

$$c(\gamma;\eta) := \beta \int_{\Omega} \gamma \cdot \eta \, dx \qquad \beta = -t^2/(1-\alpha t^2) \tag{4}$$

The strain is $\varepsilon = \text{sym}(\nabla \vartheta)$, the elasticity operator is defined by $\mathbb{C}\varepsilon = \frac{1}{12}\frac{\lambda}{\mu k}\text{tr }\varepsilon I + \frac{1}{6k}\varepsilon (\mu, \lambda \text{ Lamè-Constants })$. The continuous problem now reads: Find $(w, \vartheta, \gamma) \in H_0^1(\Omega) \times H_0^1(\Omega)^2 \times L^2(\Omega)^2$ such that

$$a(w,\vartheta;v,\varphi) + b(v,\varphi;\gamma) = \int_{\Omega} f v \, dx \tag{5}$$

$$b(w,\vartheta;\eta) + c(\gamma;\eta) = 0 \qquad \text{for all } (v,\varphi,\eta) \in H^1_0(\Omega) \times H^1_0(\Omega)^2 \times L^2(\Omega)^2.$$
(6)

We consider a regular triangulation \mathcal{T} of Ω with discrete spaces of \mathcal{T} -piecewise polynomials of degree $\leq k(k \in \mathbb{N})$

$$\mathcal{P}_{k}(\mathcal{T}) := \left\{ u \in L^{2}(\Omega) \mid \forall T \in \mathcal{T}, u \mid_{T} \in \mathcal{P}_{k}(T) \right\} \qquad \mathcal{S}_{k}(\mathcal{T}) := \mathcal{P}_{k}(\mathcal{T}) \cap H^{1}_{0}(\Omega)$$
(7)

We choose here $H_w \times H_\vartheta \times L_\gamma = S_2(\mathcal{T}) \times S_2(\mathcal{T})^2 \times \mathcal{P}_0(\mathcal{T})^2$ as in [6]. The discrete problem now reads: Find $(W, \Theta, \Gamma) \in H_w \times H_\vartheta \times L_\gamma$ such that

$$a(W,\Theta;V,\Phi) + b(V,\Phi;\Gamma) = \int_{\Omega} fV \, dx \tag{8}$$

$$b(W,\Theta;H) + c(\Gamma;H) = 0 \qquad \text{for all } (V,\Phi,H) \in H_{w} \times H_{\vartheta} \times L_{\gamma}.$$
(9)

2. Stabilization Techniques



Fig. 1: Transversale displacement W of the plate with $\alpha = 1$ and $\alpha = 1/h_T^2$ In [1] is shown, that (5-6) lead asymptotically to a stable and locking free finite element discretisation. Here and in other theoretical contributions the stabilization parameter α is set equal 1 [3]. But in practical use – with coarse meshes – the choice of α has an essential influence on the solution quality. If α is too small, spurious modes appears. If it is too large, the system is too stiff. Trying to find an optimal α we made several numerical investigations. Here results are given for an all side clamped plate of $1 \cdot 1 \cdot 0.001$ m under a unit load f = 1000N/m³, $\mu = 4.2$ N/m², $\lambda = 3.6$ N/m². It is meshed with $4 \cdot 4$ squares, each divided in 2 triangels (32 fe, 211 dof). If $\alpha = 1$ the fe-solution is unusable (Fig. 1). Best approximation we get with $\alpha = 5...50$, here the maximal transverse displacement is $w_{max} = 1.2$ mm (analytical solution: $w_{max} = 1.2$ 6mm). If e.g. $\alpha = 5000$ we get $w_{max} = 0.34$ mm. Due to our numerical expierience we set α mesh dependent. With $h_T = \text{diam}(T)$ we recommend

$$\alpha = h_T^{-2} \tag{10}$$

which is in the example $\alpha = 8$. (10) corresponds to similar results in [5] and [6].

3. A Posteriori Error Estimation

Let $(w, \vartheta, \gamma) \in H_0^1(\Omega) \times H_0^1(\Omega)^2 \times L^2(\Omega)^2$ solve (5-6) and suppose that (W, Θ, Γ) satisfies (8-9) for all $(V, \Phi, H) \in H_w \times H_\vartheta \times L_\gamma$. Then, there exists a positive constant C which is independent of t, h_T and α such that we have

$$\|\vartheta - \Theta\|_{H^{1}_{0}(\Omega)} + \|w - W\|_{H^{1}_{0}(\Omega)} + \|\gamma - \Gamma\|_{H^{-1}(\operatorname{div};\Omega)} + t\|\gamma - \Gamma\|_{L^{2}(\Omega)} \le C(\sum_{T \in \mathcal{T}} \eta_{T}^{2})^{1/2}.$$
 (11)

For each element $T \in \mathcal{T}$ we define our error indicator η_T (\mathcal{E} edges of triangulation $\mathcal{E} \subset \partial \mathcal{T}$)

$$\eta_T^2 := h_T^2 \int_T |\operatorname{div}(\alpha(\nabla W - \Theta)) + \operatorname{div}\Gamma + f|^2 \, d\, x + h_T^2 \int_T |\operatorname{div}C\varepsilon(\Theta) + \alpha(\nabla W - \Theta)|^2 \, d\, x + \int_T |\nabla W|^2 \, d\, x + \frac{h}{t} \int_T |\nabla \Theta|^2 \, d\, x + \sum_{E \in \mathcal{E}} (h_E \int_E |[\alpha(\nabla W - \Theta) + \Gamma] \cdot n_E]|^2 ds + h_E \int_E |[C\varepsilon(\Theta)] \cdot n_E|^2 ds)$$
(12)

The proof of (11) is given in [4], we will just mention, that – besides the standard a posteriori arguments – we need precise mapping properties of the weak formulation, *t*-dependent interpolation spaces is in [2] and an estimate of $\dot{B}_2^{1/2}(\mathcal{T})$ due to Tartar. Concluding we will show some results of our adaptive mesh refinement algorithm.

Fig. 2: All side clamped L-shaped domain (load and material as above) after 3 and 5 automatic refinement steps controlled by (12)

We see the expected refinement towards the corner. Here and in further examples (12) designs resonable adaptive refined meshes.





4. References

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